

Scaling frequency of channel-forming flows in snowmelt-dominated streams

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[1] The scaling properties of channel-forming flows are investigated using a regional flow frequency model developed for snowmelt-dominated streams in Colorado. The model is derived from analyses of daily flow records at 32 gauging stations where we have independent measurements of the bankfull discharge. The study sites are located in alpine/subalpine basins with drainage areas ranging from 4 to 3700 km². The frequency distribution of daily flows at these locations can be reproduced with a broken power law (BPL) function described by two free parameters. Both parameters are strongly correlated with drainage area, and based on these correlations, a regional model capable of predicting the frequency of daily flows above the mean annual flow was formulated. The applicability of the model was tested using daily flow records from 32 similar-size basins in Idaho. The frequency distributions of daily flows in snowmelt-dominated streams in Colorado and Idaho with highly predictable hydrographs (i.e., 1 year autocorrelation above 0.7) are well fitted by the BPL function. According to the model, the frequency of flows greater than bankfull decreases downstream from about 15 d/yr in headwater reaches to about 6 d/yr in downstream reaches. These results imply that the basin response to precipitation and runoff is nonlinear. This multiscaling behavior can be physically interpreted as the result of scale-dependent variations in runoff and sediment supply, which influence downstream trends in the bankfull channel geometry and intensity of sediment transport.

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1. Introduction

[2] Bankfull discharge is an important index of fluvial-hydraulic processes in alluvial channels. At these flow levels, water is beginning to spill onto the floodplain; thus, bankfull discharge represents the point of incipient flooding [Williams, 1978]. In addition, flows at or near bankfull can generate sufficient shear stress to entrain sediment from the bed and banks; hence, the bankfull discharge is described as the channel-forming flow [Richards, 1982]; other terms, such as the dominant discharge [Knighton, 1998], or the discharge that is most effective for channel maintenance [Dunne and Leopold, 1978], are used synonymously. However, it is now recognized that alluvial channels are formed and maintained by a range of flows with different frequencies and that single-valued indices of channel-forming processes provide only part of the information needed to quantify sediment or water quality loads or to assess processes such as nutrient retention and habitat maintenance.

[3] In streams that are gauged, the frequency of bankfull discharge can be determined directly from statistical analy-

ses of the gauge record of peak flows or daily flows. Results from peak-flow frequency analyses of streams in a wide range of hydroclimatic settings indicate that the recurrence interval of bankfull discharge typically falls in the range of 1–3 years on the annual series [Wolman and Leopold, 1957; Wolman and Miller, 1960; Emmett, 1975; Whiting et al., 1999; Castro and Jackson, 2001; Chaplin, 2005; Dodov and Fofoula-Georgiou, 2005; Keaton et al., 2005; Sherwood and Huitger, 2005; Mulvihill et al., 2009; Rachol and Boley-Morse, 2009]. Exceptions to this “rule of thumb” have been noted in several studies, notably those of Williams [1978], Pickup and Warner [1976], and DeRose et al. [2008]. Fewer studies have examined the duration of bankfull flow, expressed as the percentage of time that this discharge is equaled or exceeded. Here, there appears to be a much wider range in reported values. Empirical relations developed for gravel bed channels in Colorado and Idaho suggest that the duration of the bankfull discharge varies from ~0.4 to 24 d/yr [Andrews, 1984; Pitlick and Van Steeter, 1998; Whiting et al., 1999; Torizzo and Pitlick, 2004]. Dodov and Fofoula-Georgiou [2005] examined links between channel geometry and flow frequency using data from dozens of gauging stations in the midwestern U. S. A. and found that the exceedance frequency of bankfull discharge increased systematically downstream, from about 0.3 d/yr in channels draining headwater basins (<10² km²) to about 7 d/yr in channels draining larger areas (>10⁴ km²). Whereas the results presented by Torizzo and Pitlick [2004] and Whiting et al. [1999] showed no clear relation between

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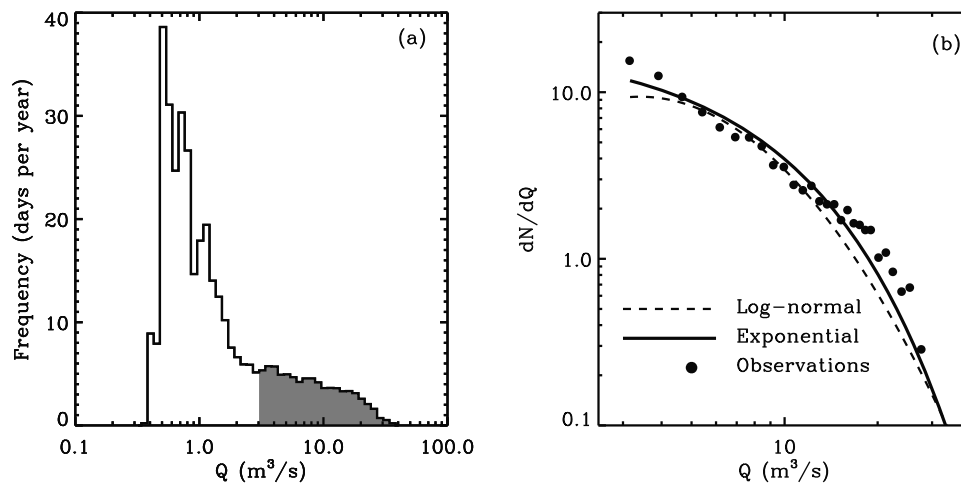


Figure 1. Frequency of daily flows of the Williams Fork River, Colo, USGS station 09036000. (a) Panel shows a histogram of daily flows; the shaded area corresponds to the range of flows analyzed in this paper. (b) Panel shows the observed frequency of daily flows and corresponding fits of the lognormal and exponential probability density functions. Figure 1b shows the frequency with which a flow range is observed is given by the integral of dN/dQ . Therefore, the y-axis (dN/dQ) has units of $\text{d yr}^{-1} \text{Q}^{-1}$.

bankfull discharge duration and drainage basin area, the results presented by *Dodov and Foufoula-Georgiou* [2005] suggest that streamflow statistics may be strongly influenced by scale-dependent changes in channel geometry and morphology.

[4] In this paper we develop a regional model to investigate the scaling properties of channel-forming flows in streams and rivers in Colorado where we have independent measurements of the bankfull discharge, Q_{bf} . The first section of the paper focuses on the development and performance of three candidate functions for fitting the distribution of daily flows in snowmelt-dominated streams. We then seek scaling relations between the parameters of the preferred distribution function and selected drainage basin characteristics, with the goal of establishing a regional model to predict the distribution of daily flows at ungauged sites. Similar to the study by *Fennessey and Vogel* [1990], we focus on the distribution of flows above the mean annual flow, Q_{ma} , which carry 80% of the annual discharge and nearly all of the bed load in gravel bed channels [*Andrews*, 1994; *Whiting et al.*, 1999; *Torizzo and Pitlick*, 2004; *Mueller and Pitlick*, 2005]. The regional model is then tested using corresponding data from similar-size streams and rivers in Idaho. The remainder of the paper focuses on scale-dependent variations in model parameters and analysis of suggested links between flow frequency and downstream trends in runoff and channel geometry.

2. Characterization of the Distribution of Daily Flows

[5] The approach commonly used for formulating daily flow frequency distributions at ungauged locations is to first identify a function that describes the frequency of flows at gauged locations, then develop a scaling relation based on drainage basin characteristics to transfer the function to ungauged locations. The extension of flow records from gauged to ungauged sites is straightforward in cases where the observed distribution has a simple statistical form, and

the parameters of that distribution scale linearly over a range of drainage areas. However, frequency distributions of daily flows are often complex (e.g., bimodal) and potentially difficult to model with a simple function. *Vogel and Fennessey* [1995] note that daily streamflows originate from highly skewed populations, and sample estimates of distribution parameters may contain large bias even for sites with very long records. A number of different statistical functions have been used to model daily flow distributions, including the lognormal, exponential, γ , and pareto [*Fennessey and Vogel*, 1990; *Vogel and Fennessey*, 1993; *Nash*, 1994; *Potter*, 2001; *Vogel et al.*, 2003; *Castellarin et al.*, 2004; *Goodwin*, 2004; *Doyle et al.*, 2005; *Mueller and Pitlick*, 2005; *Archfield et al.*, 2007; *Doyle and Shields*, 2008]. The procedure for fitting these distributions is straightforward, and any one of them might offer a reasonably good fit to a series of observations. However, in our experience, none of these conventional distributions appears to fit the distribution of daily flows in snowmelt-dominated streams particularly well. Figure 1 shows the frequency distribution of daily flows for the Williams Fork River, Colo (U.S. Geological Survey [USGS] gauge 09036000), for which 73 years of record are available. Figure 1a shows that the distribution is strongly skewed and there are two modes, one reflecting base flows, the other reflecting snowmelt flows. In this case it would be difficult to model the entire distribution with a single function. Figure 1b presents a subset of the distribution with the frequencies normalized by the bin size and plotted with logarithmic scales to emphasize the behavior of intermediate to high flows. Figure 1b also shows the fit of the lognormal and the exponential functions. Unfortunately, neither of these functions fit the data particularly well. The lognormal function provides a poor fit across the entire range of flows. The exponential function provides a better fit to intermediate flows, but it underestimates both high and low flows. The observed flow distribution appears to fall as a straight line in the log-log plot, suggesting a power law behavior [*Newman*, 2005], but at a

Table 1. Hydrologic Characteristics of the Selected Streams in Colorado, Length of Record Available, and Mean Basin Elevation, from *Richter et al.* [1984]^a

| USGS Gauge Number | Name | <i>n</i> (years) | DA (km ²) | <i>Q</i> _{br} (m ³ /s) | BE (m) | <i>d.f.</i> | χ^2 | | | χ^2_v | | | <i>a</i> ₀ | <i>a</i> ₁ | <i>R</i> (1y) |
|-------------------------|-----------------------|---------------------|--------------------------|---|-----------|-------------|----------|-------|------|------------|-----|-----|-----------------------|-----------------------|---------------|
| | | | | | | | LN | EXP | BPL | LN | EXP | BPL | | | |
| 06614800 | Michigan River | 33 | 4.0 | 0.70 ^b | 3444 | 21 | 69.7 | 50.8 | 41.7 | 3.32 | 2.4 | 2.0 | 35.2 | 0.7 | 0.77 |
| 06721500 | North Saint Vrain | 16 | 84.0 | 11.6 ^c | | 32 | 24.2 | 18.4 | 15.5 | 0.76 | 0.6 | 0.5 | 57.5 | 8.8 | 0.87 |
| 06725500 | Middle Boulder Creek | 87 | 93.8 | 9.50 ^d | 3170 | 34 | 108.4 | 90.8 | 30.3 | 3.19 | 2.7 | 0.9 | 52.5 | 9.5 | 0.82 |
| 07083000 | Halfmoon Creek | 60 | 61.1 | 7.00 ^c | 3597 | 31 | 72.2 | 29.5 | 25.4 | 2.33 | 1.0 | 0.8 | 49.4 | 5.3 | 0.79 |
| 07086500 | Clear Creek | 49 | 174 | 23.6 ^c | 3597 | 26 | 52.5 | 20.8 | 28.5 | 2.02 | 0.8 | 1.1 | 46.4 | 12.6 | 0.73 |
| 09010500 | Colorado River | 53 | 138 | 14.3 ^c | 3231 | 40 | 77.9 | 43.8 | 28.6 | 1.95 | 1.1 | 0.7 | 36.3 | 14.9 | 0.74 |
| 09022000 | Fraser River | 27 | 27.2 | 2.69 ^b | | 25 | 64.4 | 43.0 | 28.0 | 2.58 | 1.7 | 1.1 | 43.0 | 2.7 | 0.81 |
| 09035700 | Williams Fork | 41 | 90.6 | 10.5 ^c | 3444 | 38 | 70.2 | 48.5 | 18.8 | 1.85 | 1.3 | 0.5 | 33.1 | 9.8 | 0.77 |
| 09035800 | Darling Creek | 41 | 22.7 | 1.83 ^d | 3292 | 28 | 43.6 | 25.3 | 29.9 | 1.56 | 0.9 | 1.1 | 36.3 | 2.0 | 0.79 |
| 09035900 | South Williams Fork | 41 | 70.3 | 7.00 ^c | 3322 | 35 | 55.6 | 29.9 | 18.6 | 1.59 | 0.9 | 0.5 | 41.1 | 6.5 | 0.83 |
| 09036000 | Williams Fork | 73 | 231 | 20.1 ^c | 3322 | 36 | 108.4 | 58.2 | 40.9 | 3.01 | 1.6 | 1.1 | 38.6 | 21.4 | 0.78 |
| 09047500 | Snake River | 58 | 149 | 12.1 ^d | 3475 | 33 | 88.9 | 38.3 | 37.1 | 2.70 | 1.2 | 1.1 | 47.1 | 11.5 | 0.78 |
| 09052400 | Boulder Creek | 28 | 22.2 | 1.12 ^d | 3444 | 30 | 73.7 | 53.1 | 27.9 | 2.46 | 1.8 | 0.9 | 47.9 | 3.1 | 0.83 |
| 09055300 | Cataract Creek | 28 | 31.1 | 1.41 ^d | 3292 | 31 | 45.8 | 33.5 | 20.8 | 1.48 | 1.1 | 0.7 | 39.2 | 4.6 | 0.80 |
| 09058500 | Piney River | 48 | 33.7 | 2.43 ^d | 3322 | 28 | 95.0 | 68.8 | 24.6 | 3.39 | 2.5 | 0.9 | 40.4 | 5.3 | 0.76 |
| 09058800 | East Meadow Creek | 40 | 9.4 | 0.58 ^d | 3139 | 26 | 136.0 | 104.9 | 82.9 | 5.23 | 4.0 | 3.2 | 29.3 | 1.1 | 0.76 |
| 09060500 | Rock Creek | 28 | 123 | 6.20 ^c | 2865 | 32 | 59.5 | 42.4 | 21.0 | 1.86 | 1.3 | 0.7 | 32.1 | 8.1 | 0.72 |
| 09063200 | Wearyman Creek | 42 | 22.7 | 0.84 ^d | 3322 | 31 | 56.7 | 42.3 | 48.1 | 1.83 | 1.4 | 1.6 | 34.1 | 2.0 | 0.79 |
| 09063400 | Turkey Creek | 43 | 61.6 | 2.69 ^d | 3292 | 30 | 38.6 | 19.3 | 22.4 | 1.29 | 0.6 | 0.7 | 35.2 | 5.1 | 0.77 |
| 09066000 | Black Gore Creek | 52 | 32.6 | 3.29 ^d | 3292 | 36 | 77.8 | 47.6 | 15.8 | 2.16 | 1.3 | 0.4 | 30.7 | 4.7 | 0.78 |
| 09066100 | Big Horn Creek | 43 | 11.8 | 1.04 ^d | 3383 | 28 | 68.3 | 38.2 | 23.6 | 2.44 | 1.4 | 0.8 | 40.4 | 2.1 | 0.78 |
| 09066150 | Pitkin Creek | 40 | 13.8 | 2.44 ^d | 3353 | 18 | 59.4 | 35.6 | 31.8 | 3.30 | 2.0 | 1.8 | 41.7 | 2.3 | 0.76 |
| 09066200 | Booth Creek | 42 | 15.6 | 3.27 ^d | 3322 | 28 | 66.9 | 35.8 | 19.6 | 2.39 | 1.3 | 0.7 | 36.3 | 2.9 | 0.76 |
| 09066400 | Red Sandstone Creek | 43 | 19.0 | 3.22 ^d | 3261 | 26 | 70.6 | 38.9 | 16.6 | 2.71 | 1.5 | 0.6 | 30.2 | 2.5 | 0.75 |
| 09074800 | Castle Creek | 25 | 83.4 | 4.45 ^d | 3505 | 34 | 34.6 | 15.0 | 17.6 | 1.02 | 0.4 | 0.5 | 51.7 | 7.0 | 0.85 |
| 09078100 | N Fork Fryngpan River | 17 | 31.1 | 3.17 ^b | 3475 | 32 | 29.4 | 20.2 | 11.4 | 0.92 | 0.6 | 0.4 | 35.8 | 4.8 | 0.73 |
| 09078200 | Cunningham Creek | 17 | 18.4 | 2.52 ^b | 3261 | 31 | 31.7 | 24.3 | 22.7 | 1.02 | 0.8 | 0.7 | 27.5 | 3.0 | 0.73 |
| 09081600 | Crystal River | 51 | 433 | 49.0 ^b | 3109 | 33 | 105.0 | 90.5 | 34.0 | 3.18 | 2.7 | 1.0 | 46.4 | 52.9 | 0.80 |
| 09112500 | East River | 84 | 749 | 37.5 ^b | 3109 | 30 | 118.3 | 80.4 | 27.4 | 3.94 | 2.7 | 0.9 | 47.1 | 59.5 | 0.79 |
| 09124500 | Lake Fork | 69 | 865 | 42.0 ^b | 3322 | 41 | 104.8 | 47.5 | 28.4 | 2.56 | 1.2 | 0.7 | 50.1 | 40.8 | 0.79 |
| 09242500 | Elk River | 36 | 1075 | 101 ^b | 2560 | 38 | 47.8 | 51.5 | 33.4 | 1.26 | 1.4 | 0.9 | 46.4 | 105.9 | 0.84 |
| 09244410 | Yampa River | 21 | 3700 | 167 ^b | 2621 | 31 | 37.4 | 42.2 | 28.9 | 1.21 | 1.4 | 0.9 | 44.3 | 209.7 | 0.82 |

^a*n*, length of record available; BE, Mean Basin Elevation; *d.f.*, degrees of freedom; χ^2 and reduced χ^2 (χ^2_v) of the fits of the lognormal (LN), exponential (EXP), and broken power law (BPL) functions to the daily flow distribution, parameters, *a*₀ and *a*₁, of the fit of the broken power law function, and annual autocorrelation (*R*(1y)) of daily flows.

^bReference for *Q*_{br}: *Andrews* [1984].

^cReference for *Q*_{br}: *Torizzo and Pitlick* [2004].

^dReference for *Q*_{br}: *Surian and Andrews* [1999].

discharge of approximately 20 m³/s, there is an inflection in the distribution and the slope appears to steepen.

2.1. Distribution Functions

[6] In this section we introduce three probability density functions for characterizing the distribution of daily flows in snowmelt-dominated streams. The first two functions, the lognormal (LN) and the exponential (EXP), have been used previously to model the distribution of flows within a specific range, such as flows below the median discharge [*Fennessey and Vogel*, 1990], or flows above one half the bankfull discharge [*Mueller and Pitlick*, 2005]. The third function is termed the broken power law (BPL). To our knowledge, this function has not been used to characterize statistical distributions in hydrology; however, it has been used in other disciplines to interpret transitions (breaks) in power law behavior above some characteristic scale. This transition is reflected by a break in slope of logarithmic plots of event frequency versus event size [*Newman*, 2005], giving a continuous distribution with two separate straight-line segments. The inflection in the distribution suggests that the data either do not follow a power law or that there is an upper limit to the size of events [*Burroughs and Tebbens*, 2001; *Aban et al.*, 2006].

[7] The lognormal probability density function is

$$\frac{dN}{dQ} = \frac{1}{Q\sigma\sqrt{2\pi}} \exp\left[-\frac{(\ln(Q) - \mu)^2}{2\sigma^2}\right], \quad (1)$$

where *N* is the number of days per year, *Q* is the discharge, and μ and σ are location and scale parameters, respectively. These two parameters correspond to the mean and the standard deviation of the natural logarithm of *Q* when the whole range of flows is considered.

[8] The exponential probability density function is

$$\frac{dN}{dQ} = b_0 e^{-b_1 Q}, \quad (2)$$

where *b*₀ is a normalization parameter and *b*₁ is the slope of the discharge-frequency relation. This distribution is simple, but as mentioned above, it fails to reproduce the frequency distribution of low and high flows in some cases.

[9] The broken power law probability density function is

$$\frac{dN}{dQ} = \frac{a_0/a_1}{(Q/a_1)^\alpha + (Q/a_1)^\beta}, \quad (3)$$

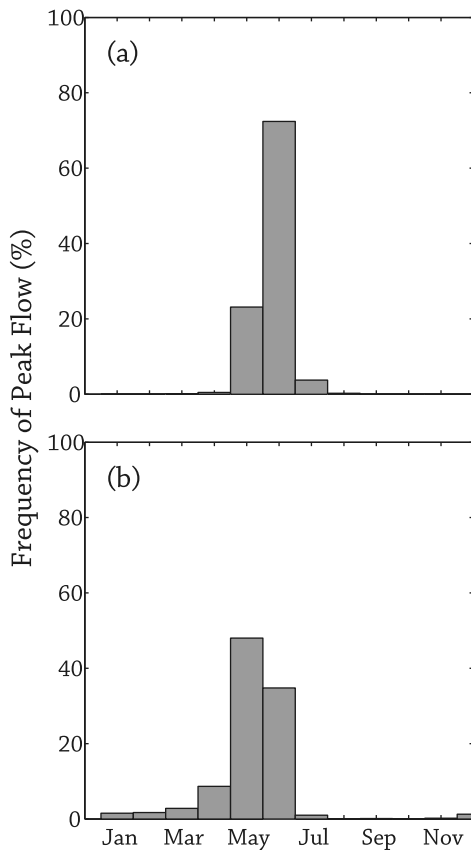


Figure 2. Seasonal distributions of peak flows at (a) 32 sites in Colorado and (b) 32 sites in Idaho.

where a_0 is a normalization parameter, a_1 is the value of Q corresponding to the inflection point of the distribution, and α and β are parameters that set the slopes of the two separate segments of the frequency distribution formed by intermediate to high flows. It is within this range of flows that we see a distinct change in the shape of the frequency distribution, which presumably reflects an upper bound on the intensity of runoff produced in alpine-subalpine basins.

2.2. Data Sources

[10] Thirty-two high-elevation basins in Colorado were selected for this analysis. These basins are located in the southern Rocky Mountains, with drainage areas ranging from 4 to 3700 km² (Table 1). The watersheds are generally forested with vegetation dominated by Engelman spruce (*Picea engelmannii*), subalpine fir (*Abies lasiocarpa*), and lodgepole pine (*Pinus contorta*). Most of the basins have been glaciated, and glacial deposits are found down to about 2400 m in elevation [Surian and Andrews, 1999]. At least 60% of the annual precipitation in the region falls as snow during the winter and spring months [Serreze et al., 1999]. Peak flows in high-elevation basins in Colorado are generated almost exclusively by snowmelt (Figure 2). Rainfall from convective storms can generate significant peaks at lower elevations (<2300 m); however, our analysis is restricted to drainage basins with mean elevations above 2500 m (Table 1) where runoff from convective storms is much lower than the peaks produced by snowmelt [Jarrett and Costa, 1988; Jarrett, 1990; Pitlick, 1994]. The study sites were selected

on the basis of four criteria: (1) minimum flow regulation, (2) daily flow records longer than 16 complete water years, (3) availability of data regarding bankfull discharge, Q_{bf} , and (4) a wide range of drainage areas (Table 1).

[11] The bankfull discharge for each site in Colorado was estimated on the basis of field surveys of channel geometry and water-surface elevation taken over a range of flows. In general, the bankfull flow level could be identified in the field by a distinct break in slope between the channel and the floodplain and by changes in vegetation. Site surveys were conducted by different investigators; however, the methods used in estimating bankfull discharges were similar. Andrews [1984] and Surian and Andrews [1999] surveyed cross sections and bankfull elevations through each of their study reaches and correlated those measurements with the observed stage-discharge relation for the gauging station. Torizzo and Pitlick [2004] surveyed 9–12 cross sections at each of their study sites and estimated bankfull discharge by direct observation and correlation with gauging station records. Segura [2008] surveyed 13–18 cross sections at each study site and estimated bankfull discharge by calibrating a 2-D flow model to minimize differences between modeled water-surface elevations and surveyed bankfull elevations. The estimates of bankfull discharge for study sites in Colorado range from 0.6 to 167 m³/s.

[12] At gauged locations, the bankfull discharge can sometimes be identified graphically by a change in the stage-discharge relation (or area-width relation) [Williams, 1978]. We plotted stage-discharge relations for a dozen sites in Colorado and found that in all cases gauge height changes rather slowly with increasing discharge, and often, there is little visible change in the stage-discharge relation at bankfull flow (Figure 3). The asymptotic trends in gauge height in these streams are a reflection of both hydraulics and hydrology. Channel hydraulics govern the rate that gauge height (or depth) changes with discharge, and this commonly results in a curved stage-discharge relation. Snowmelt hydrology adds to this effect by limiting the intensity of runoff produced from a melting snowpack [Cline, 1997; Pitlick, 1994]. Both effects become important in our interpretation of the results presented later.

[13] We tested the general applicability of the daily flow models using similar measurements of flow and drainage basin characteristics in 32 unregulated streams and rivers in Idaho. The sites in Idaho are located in mountainous areas in the central portion of the state, with drainage areas ranging from 15 to 2694 km² (Table 2). These basins are mostly forested, although the density of forest cover decreases from north to south and with elevation. Bedrock lithologies are dominated by intrusive rocks of the Idaho batholith [King et al., 2004; Mueller et al., 2005] and headwater areas in most basins have been glaciated. Mean annual precipitation in the region varies from 30 to 150 cm, with >60% of precipitation falling as snow [Serreze et al., 1999]. Similar to Colorado, peak flows are generated primarily by spring snowmelt (Figure 2).

[14] For most of the study sites, we retrieved period-of-record mean daily discharge values from the U.S. Geological Survey National Water Information System (<http://waterdata.usgs.gov/nwis>). For several sites which are maintained by the U.S. Forest service (Table 2), we obtained daily flow values from the Boise Adjudication Team Web site (<http://www.fs.fed.us/rm/boise/research/watershed/BAT/index.shtml>), described by King et al. [2004]. We were not

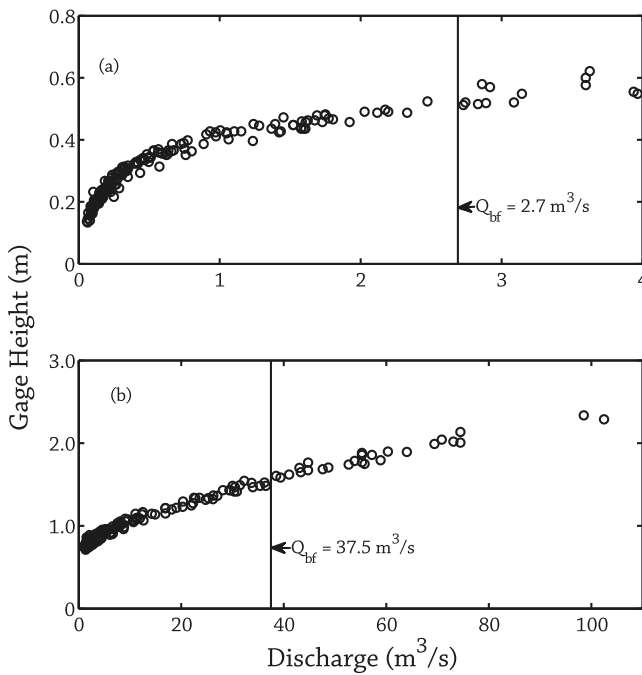


Figure 3. Stage-discharge relations for (a) Fraser River near Winter Park, Colo, USGS station 09022000, drainage area = 27.2 km², and (b) East River at Almont, Colo, USGS station 09112500, drainage area = 749 km².

involved in field data collection at the sites in Idaho; however, published descriptions of the field methods [Castro and Jackson, 2001; King et al., 2004] indicate that bankfull discharges were estimated using techniques similar to those described above. Bankfull discharges of these streams and rivers vary from 2.5 to 326 m³/s (Table 2).

2.3. Data Analysis

[15] Frequency distributions of daily flows ranging from the mean annual flow to the maximum observed discharge were constructed for each site by dividing the observations into 50 equal intervals of discharge, Q . The number of values, N_i , falling in each interval was counted, and those values were normalized by the bin width, dQ , to produce a frequency distribution where the number of observations is then independent of interval width [Newman, 2005]. In some cases, the number of days in a given discharge interval fell below 10; hence, two consecutive bins were joined. This only occurred for very high discharges and was done in order to ensure that the uncertainty in the number of values in each bin approximated a normal distribution [Bevington and Robinson, 2003; Press et al., 2007; Segura, 2008]. The average frequency with which a flow is observed within a given interval of discharge, Q_i to Q_{i+1} , is thus

$$\int_{Q_i}^{Q_{i+1}} \frac{dN}{dQ} dQ, \quad (4)$$

where dN/dQ has the units of $\text{d yr}^{-1} \text{Q}^{-1}$.

[16] The parameters of the flow frequency distributions (EXP, LN, and BPL) were estimated by simultaneously varying individual combinations of the two parameters, over

a uniform grid of more than 62,000 values and finding the parameter set that yielded the lowest overall χ^2 score [Bevington and Robinson, 2003; Press et al., 2007]. χ^2 scores were calculated using a weighted sum of the squared differences between the observed and calculated frequencies of discharge,

$$\chi^2 = \sum_{i=1} [f_i - f(Q_i)]^2 / \sigma_i^2, \quad (5)$$

where σ_i is the uncertainty associated with the observed frequencies, f_i . The χ^2 statistic is not used here to perform a rigorous goodness-of-fit test, since we are not attempting to fit the entire distribution of daily flows (only those above the mean), and we are not presuming that the observations forming the distributions are statistically independent. Daily flows are serially correlated; thus, there are inherent limitations in using standard goodness-of-fit tests to compare the observed frequency distributions to a set of hypothetical distributions. In this case, the distributions EXP, LN, and BPL are used only as mathematical functions for fitting the observations, and the χ^2 scores are not used as the basis for hypothesis testing. The parameter fits and resulting χ^2 scores simply provide a numerical value upon which we can index the overall performance of different models. To our knowledge there is no comparable parameter-fitting method or statistical test that gets around the problem of serial correlation in daily flows.

[17] Preliminary fits of the data using minimum χ^2 scores indicated that two out of the four parameters of the BPL distribution could be set to constant values. The average best fit values for the parameters α and β were 1.00 ± 0.03 and 6.98 ± 0.99 ; hence, these two parameters were set to 1 and 7, respectively. The decrease of χ^2 obtained by allowing all the four parameters to vary is small and does not justify the addition of two extra parameters with respect to the fit performed by holding α and β constant.

[18] The exponential, lognormal, and broken power law functions described above were fitted to the daily flow frequency distribution for each of the sites in Colorado. For a statistically acceptable fit, the value of the χ^2 needs to be close to the number of degrees of freedom in the fit [Bevington and Robinson, 2003; Press et al., 2007]. When two or more different functions are compared, the one that provides the best description of the data is the one with the lowest χ^2 .

3. Results

3.1. Selection of the Best Function to Describe Daily Flows

[19] The comparison of χ^2 minima on a site-by-site basis indicated that the BPL function provided the best fit in 27 out of 32 cases (Table 1). The exponential function provided a better fit in five cases but otherwise yielded relatively high χ^2 values (Table 1). The lognormal function produced the poorest fit, yielding χ^2 values up to five times the number of degrees of freedom. Given these results, it appears that the BPL provides a better fit to the distribution of snowmelt flows than either the exponential or lognormal functions.

[20] In order to evaluate the overall capability of the models to describe the whole data set, it is useful to introduce the concept of a reduced χ^2 (χ^2_r), which is the value of

Table 2. Hydrologic Characteristics of the Selected Streams in Idaho, Length of Record Available, and Mean Basin Elevation, from *Berenbrock* [2002] and *Hortness* [2006]^a

| USGS Gauge Number | Name | <i>n</i> (years) | DA (km ²) | <i>Q</i> _{bf} (m ³ /s) | BE (m) | <i>d.f.</i> | χ^2 | χ^2_{ν} | <i>a</i> ₀ | <i>a</i> ₁ | R(1y) |
|-----------------------|---------------------------|------------------|-----------------------|--|--------|-------------|----------|----------------|-----------------------|-----------------------|-------|
| 11111111 ^b | Little Slate Creek | 46 | 162.1 | 12.2 ^c | | 27 | 32.6 | 1.2 | 80.7 | 11.4 | 0.85 |
| 11111112 ^b | Lolo Creek | 35 | 106.2 | 11.8 ^b | | 30 | 24.6 | 0.8 | 72.4 | 11.8 | 0.78 |
| 11111113 ^b | Main Fork Red River | 36 | 128.7 | 9.4 ^c | | 25 | 33.8 | 1.4 | 56.7 | 10.9 | 0.70 |
| 11111114 ^b | Rapid River | 88 | 279.7 | 17.7 ^c | | 32 | 27.4 | 0.9 | 52.5 | 22.9 | 0.83 |
| 11111115 ^b | South Fork Red River | 35 | 98.9 | 7.3 ^c | | 22 | 47.3 | 2.2 | 51.7 | 7.6 | 0.66 |
| 13120000 | NF Big Lost River | 62 | 297.1 | | 2640 | 29 | 31.8 | 1.1 | 47.1 | 17.9 | 0.74 |
| 13120500 | Big Lost River | 58 | 1165.5 | | 2618 | 34 | 28.9 | 0.8 | 44.3 | 58.4 | 0.75 |
| 13135500 | Big Wood River | 23 | 356.1 | 21.9 ^c | 2501 | 34 | 11.8 | 0.3 | 50.1 | 26.6 | 0.77 |
| 13185000 | Boise River | 96 | 2153.8 | 167 ^c | 1986 | 27 | 40.1 | 1.5 | 64.1 | 156.0 | 0.80 |
| 13186000 | SF Boise River | 61 | 1645.0 | 68.1 ^d | 2141 | 38 | 11.7 | 0.3 | 50.1 | 126.7 | 0.79 |
| 13196500 | Bannock Creek | 23 | 14.9 | | 1619 | 22 | 78.3 | 3.6 | 50.1 | 0.3 | 0.64 |
| 13200000 | Mores Creek | 56 | 1033.0 | 26.4 ^d | 1546 | 25 | 24.7 | 1.0 | 59.3 | 43.1 | 0.69 |
| 13200500 | Robie Creek | 21 | 40.9 | | 1426 | 17 | 81.7 | 4.8 | 44.3 | 1.5 | 0.59 |
| 13235000 | South Fork Payette River | 66 | 1181.0 | 86.4 ^c | 2080 | 33 | 22.1 | 0.7 | 63.1 | 109.4 | 0.83 |
| 13240000 | Lake Fork Payette River | 61 | 126.1 | 11.2 ^d | 2118 | 34 | 25.5 | 0.7 | 41.1 | 29.3 | 0.77 |
| 13251500 | Weiser River | 63 | 94.5 | | 1402 | 24 | 14.4 | 0.6 | 21.2 | 10.3 | 0.67 |
| 13258500 | Weiser River | 67 | 1567.0 | 37.6 ^d | 1417 | 14 | 154.4 | 11.0 | 79.4 | 67.4 | 0.68 |
| 13261000 | Little Weiser River | 36 | 205.0 | | 1619 | 30 | 10.8 | 0.4 | 65.1 | 15.3 | 0.77 |
| 13295000 | Valley Creek | 66 | 380.7 | 24.1 ^c | 2231 | 32 | 24.6 | 0.8 | 59.3 | 26.0 | 0.84 |
| 13295500 | Salmon River | 35 | 1297.6 | | 2373 | 36 | 14.0 | 0.4 | 64.1 | 78.2 | 0.84 |
| 13296000 | Yankee River | 27 | 505.1 | | 2436 | 33 | 36.6 | 1.1 | 39.8 | 36.6 | 0.71 |
| 13296500 | Salmon River | 72 | 2077.2 | 118 ^c | 2375 | 34 | 25.5 | 0.8 | 54.1 | 137.2 | 0.81 |
| 13297330 | Thompson Creek | 35 | 75.4 | 2.5 ^c | 2322 | 25 | 43.1 | 1.7 | 43.0 | 2.9 | 0.66 |
| 13297355 | Squaw Creek | 35 | 185.4 | 5.1 ^c | 2356 | 27 | 49.7 | 1.8 | 36.3 | 6.8 | 0.68 |
| 13298000 | Salmon River | 19 | 1377.9 | | 2467 | 21 | 36.6 | 1.7 | 48.6 | 35.1 | 0.66 |
| 13308500 | MF Salmon River | 43 | 346.5 | | 2281 | 35 | 12.0 | 0.3 | 42.3 | 45.0 | 0.80 |
| 13309220 | Middle Fork Salmon River | 16 | 2693.6 | 214 ^c | 2192 | 19 | 6.4 | 0.3 | 47.1 | 214.0 | 0.71 |
| 13310500 | SF Salmon River | 32 | 238.3 | | 2021 | 35 | 29.5 | 0.8 | 42.3 | 26.5 | 0.82 |
| 13310700 | SF Salmon River | 35 | 855.0 | 70.8 ^c | 1945 | 32 | 15.9 | 0.5 | 50.9 | 86.8 | 0.76 |
| 13311000 | EF South Frk Salmon River | 28 | 51.0 | 9.6 ^d | 2354 | 29 | 27.8 | 1.0 | 40.4 | 4.4 | 0.73 |
| 13313000 | Johnson Creek | 79 | 552.0 | 39.7 ^c | 2175 | 33 | 22.4 | 0.7 | 37.4 | 73.9 | 0.78 |
| 13337500 | SF Clearwater River | 30 | 676.0 | | 1570 | 29 | 12.7 | 0.4 | 50.9 | 46.1 | 0.77 |

^a*n*, length of record available; BE, Mean Basin Elevation; *d.f.*, degrees of freedom; χ^2 , reduced χ^2 (χ^2_{ν}), and parameters, *a*₀ and *a*₁, of the fit of the broken power law functions to the daily flow distribution, and annual autocorrelation (*R*(1y)) of daily flows.

^bFlow data obtained from *King et al.* [2004]. The gauge number is arbitrary.

^cReference for *Q*_{bf}: *King et al.* [2004].

^dReference for *Q*_{bf}: *Castro and Jackson* [2001].

the χ^2 divided by the number of degrees of freedom. The average value of χ^2_{ν} gives an estimate of how well a particular function describes the complete data set. An excellent fit should yield $\chi^2_{\nu} \leq 1$. The BPL function provides the best score with $\langle \chi^2_{\nu} \rangle = 0.95$, followed by the EXP with $\langle \chi^2_{\nu} \rangle = 1.40$ and by the LN with $\langle \chi^2_{\nu} \rangle = 2.26$ (Table 1). This result corroborates the choice of the BPL model as the functional form to describe the frequency distribution of intermediate to high daily flows in snowmelt dominated streams.

3.2. Development of a Regional Model for Colorado

[21] Figure 4 presents the fit of the BPL function to the frequency distribution of daily flows at six representative sites in Colorado. The χ^2_{ν} of the fit on these six basins varies between 0.48 and 1.12, which represents the range observed for most of the data (Table 1). The best-fit values of *a*₀ and *a*₁ for all 32 sites are plotted versus drainage area (DA), bankfull discharge (*Q*_{bf}), and mean basin elevation (BE) in Figure 5. Linear regression was used to find the best relationship between the parameters *a*₀ and *a*₁ and drainage basin characteristics. The results of this analysis indicated that neither *a*₀ nor *a*₁ was strongly correlated with BE (Table 3). Therefore, elevation was discarded as a scaling parameter. Both *a*₀ and *a*₁ were positively correlated with DA and *Q*_{bf} (Figure 5). The statistics of these regressions

indicate that *Q*_{bf} and DA have very similar predicting power for *a*₀ and *a*₁ (Table 3), which is not surprising considering that DA and *Q*_{bf} are strongly correlated (Figure 6). DA is best for describing both *a*₀ and *a*₁ (Table 3). Therefore, DA was chosen to formulate the regional model. DA is a robust descriptor of the amount of runoff occurring in a given area, and this parameter has also been observed by many others as the best scaling parameter to transfer similarity properties of hydrologic processes [e.g., *Gupta et al.*, 1994; *Vogel and Sankarasubramanian*, 2000; *Ogden and Dawdy*, 2003; *Furey and Gupta*, 2005].

[22] The regional model is described by the following equation:

$$\frac{dN}{dQ} = \frac{a_0/a_1}{(Q/a_1) + (Q/a_1)^{\gamma}}, \quad (6)$$

with the parameters *a*₀ and *a*₁ scaling as a function of the drainage area, DA:

$$a_0 = (31.04 \pm 2.7) DA^{0.061 \pm 0.019} \text{ and} \quad (7)$$

$$a_1 = (0.213 \pm 0.027) DA^{0.835 \pm 0.028}. \quad (8)$$

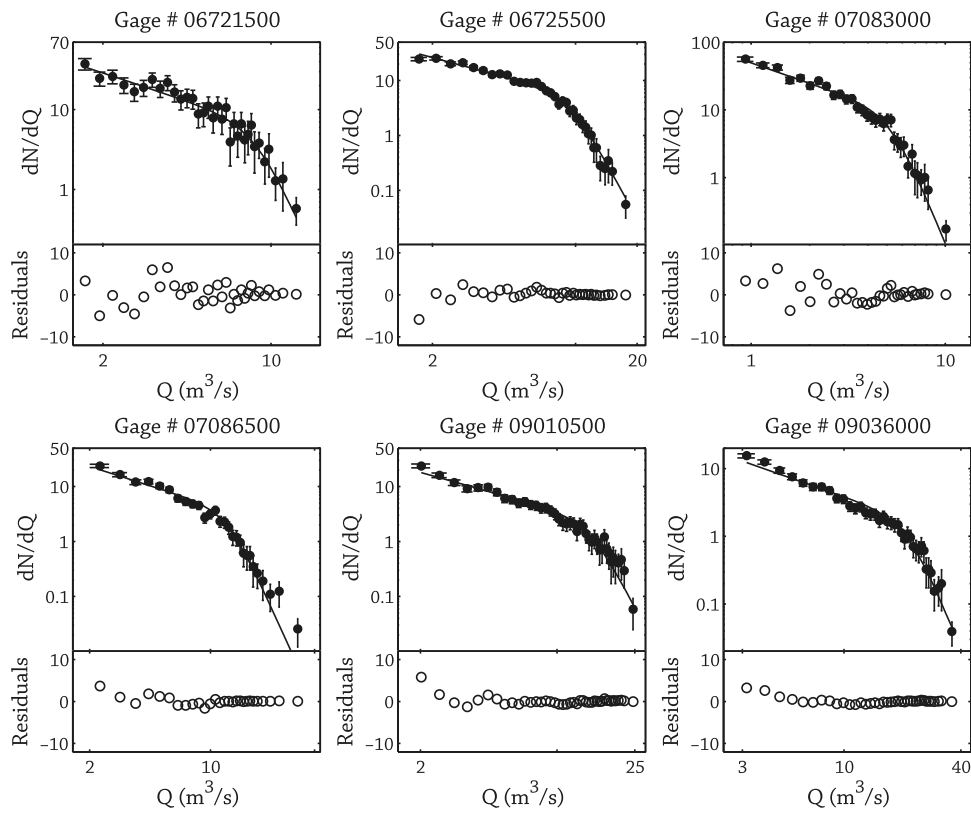


Figure 4. Fits of the broken power law (BPL) function to the daily flow frequency distributions of 6 sites in Colorado. The units of the y-axis are the same as in Figure 1b.

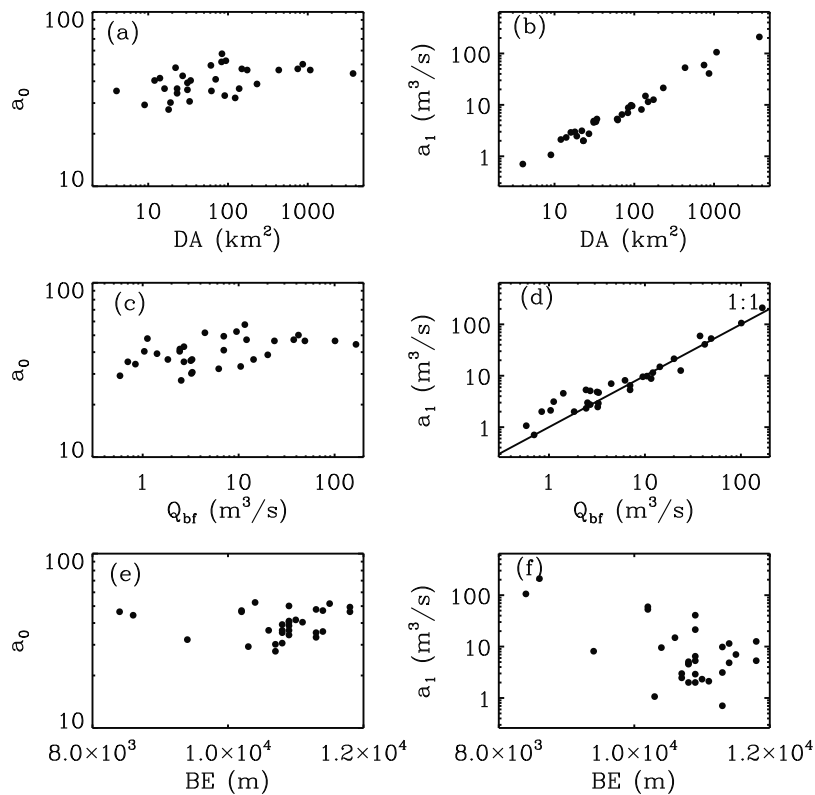


Figure 5. Parameters a_0 and a_1 of the broken power law (BPL) function versus drainage area (DA), bankfull discharge (Q_{br}), and mean basin elevation (BE) for 32 sites in Colorado.

Table 3. Summary and Statistics for Regression Between the Parameters, a_0 and a_1 , of the Broken Power Law and Drainage Area, Bankfull Flow, and Mean Basin Elevation for 32 Basins in Colorado^a

| Y | X | a | b | r^2 | P |
|----------|----------|------------------|----------------|--------|--------------|
| a_0 | DA | 31.0 (2.7) | 0.061 (0.019) | 0.243 | 0.0041 |
| a_0 | Q_{bf} | 35.8 (1.7) | 0.064 (0.021) | 0.240 | 0.0044 |
| a_0 | BE | 28.9 (118.7) | 0.0335 (0.442) | <0.001 | 0.94 |
| a_1 | DA | 0.213 (0.027) | 0.835 (0.028) | 0.966 | $\ll 0.0001$ |
| a_1 | Q_{bf} | 1.55 (0.16) | 0.867 (0.046) | 0.922 | $\ll 0.0001$ |
| a_1 | BE | [2.0 (48.2)]1E41 | -10 (2.6) | 0.341 | 0.0007 |
| a_1/DA | DA | 0.213 (0.027) | -0.165 (0.028) | 0.530 | $\ll 0.0001$ |

^a(Figure 5): a and b are parameters of the power law relation, $Y = aX^b$; values shown in parenthesis are standard errors of the regression coefficients, SE_a and SE_b ; r^2 is the coefficient of determination; and P is the significance probability.

[23] Using these relations, the number of days per year that a discharge within a particular range is observed can be computed as

$$\text{number of days} = \frac{a_0}{a_1} \int_{Q_i}^{Q_{i+1}} \frac{1}{(Q/a_1) + (Q/a_1)^7} dQ, \quad (9)$$

where a_0 and a_1 are given by equations (7) and (8).

[24] The exponent in the relation between a_1 and DA is less than one, indicating that the location of the inflection point in the distribution of daily flows does not change in direct proportion to DA. In order to investigate the scaling properties of a_1 with DA, the values of a_1 were normalized by DA and plotted versus DA (Figure 7). If the location of the inflection point between intermediate and high flows varied in proportion to drainage area, this figure would depict no correlation. Instead, Figure 7 indicates that there is an inverse correlation between a_1/DA and DA ($r^2 = 0.53$; $P \ll 0.0001$) (Table 3). This result implies that as drainage area increases the location of the inflection point, a_1 , occurs at consistently lower values of a_1/DA . In other words, it

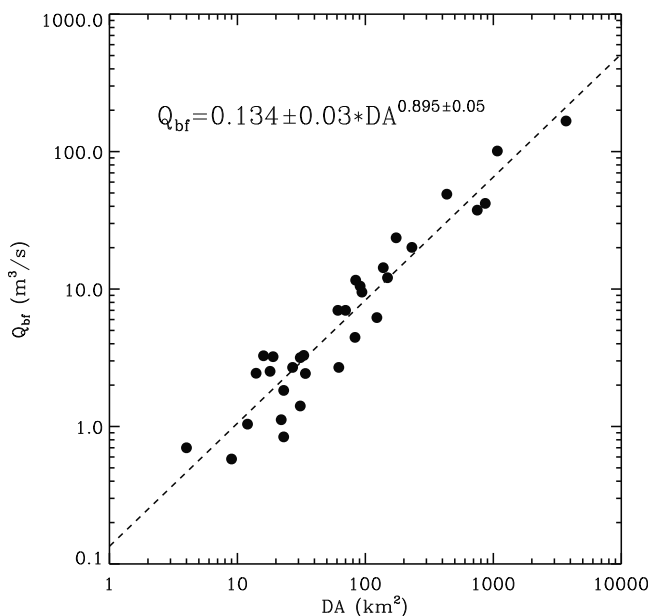


Figure 6. Relation between bankfull discharge (Q_{bf}) and drainage area (DA) for 32 sites in Colorado.

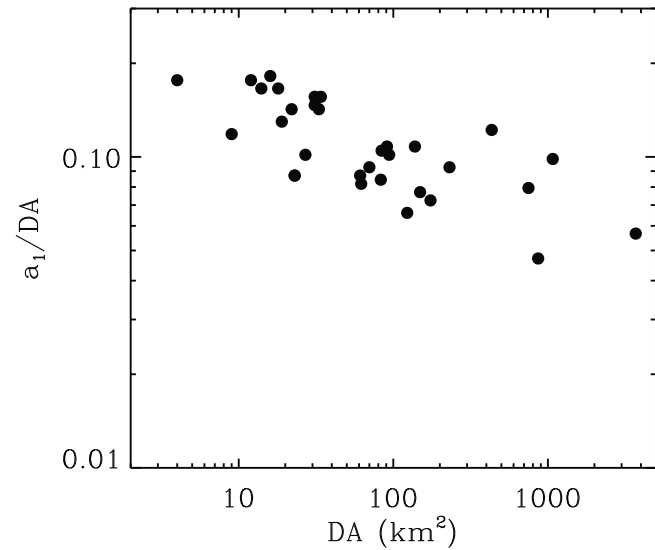


Figure 7. Relation between the parameter a_1 of the broken power law function (BPL) normalized by drainage area (DA) and DA for 32 sites in Colorado.

appears that as DA increases, the frequency of high-flow events consistently decreases downstream. We examined the relation between a_1/DA and DA for subsets of gauges within two individual drainage basins (Arkansas River and Williams Fork) and found that there was no difference in the trends within the individual basins and the trend defined for the region as a whole.

[25] The relationships shown in Figures 5–7 indicate that both the location of the inflection point in the BPL (a_1) and the magnitude of Q_{bf} grow less than linearly with DA. Furthermore, the cluster of points around the 1:1 line in Figure 5d indicates that the values of Q_{bf} and a_1 are very closely related, especially for sites with Q_{bf} above 10 m³/s and drainage areas above 120 km². The break in the frequency distribution implies that there is a shift in the relation between flow frequency and discharge at almost precisely the point where flows reach bankfull. The observed relation between a_1 and Q_{bf} suggests that in snowmelt-dominated streams the probability associated with overbank flows drops off very quickly, whereas the discharges that are important for shaping the channel (flows higher than about 67% of Q_{bf}) [Mueller *et al.*, 2005] occur almost every year.

[26] Since the flow records from the 32 sites do not encompass the same time period and they include observations in some cases going back to the early 1900s, an additional analysis was performed to ensure that the results presented above were not an expression of secular climatic variability. The same analysis was performed for a sub-sample of the Colorado data set that includes 23 gauges with overlapping daily flow records of 28 years between 1966 and 1994. Statistically indistinguishable results were found. This indicates that the behavior of the distribution parameters a_0 and a_1 is not governed by short-term variations in hydrology [Segura, 2008].

[27] The regional model described by equation (6) together with the relation between DA and Q_{bf} (Figure 6) were used to compare downstream changes in the frequency of flows between $0.2Q_{bf}$ and Q_{bf} and flows above Q_{bf} for a hypothetical high-elevation stream draining an area of 4000 km².

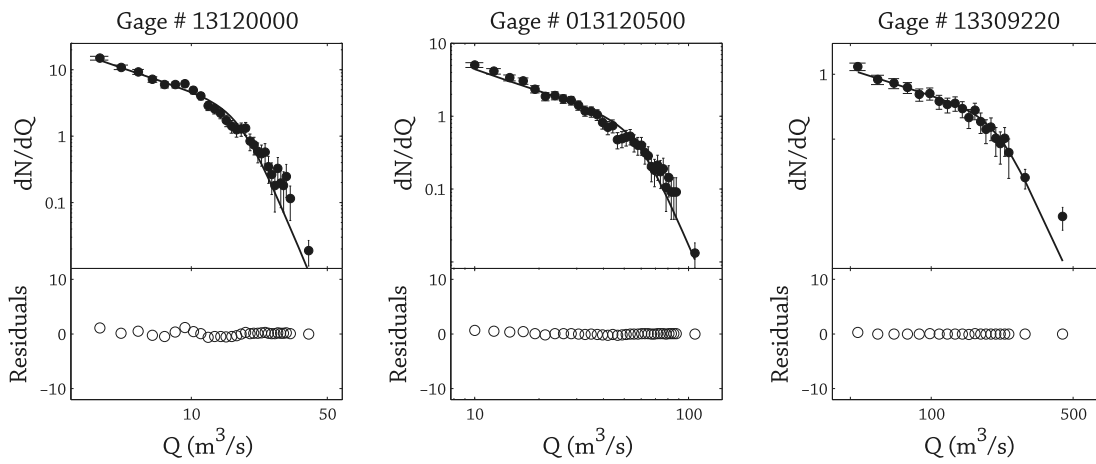


Figure 8. Fits of the broken power law (BPL) function to daily flows for three representative sites in Idaho. The units of the y-axis are the same as in Figure 1b.

The results indicate that the frequency of flows between $0.2Q_{bf}$ and Q_{bf} increases downstream from 64 d/yr in the headwaters to 81 d/yr at the mouth. In contrast, the frequency of overbank flows, $Q > Q_{bf}$, decreases downstream from 15 d/yr in the headwaters to 6 d/yr near the mouth. The downstream difference in frequency arises because the exponent in the relation between a_1 and DA (equation (8)) is less than one. Thus, even within a region of relatively homogeneous climate and land cover, the frequency of bankfull discharge appears to decrease downstream.

[28] The observation that basins with small drainage area experience more frequent high flows, but less frequent intermediate flows than sites with large drainage areas suggests that daily flow frequency distributions of snowmelt-dominated streams in Colorado exhibit multiscaling properties. Streams draining small basins are characterized by flow frequency distributions with larger standard deviations (larger tails in both extremes) compared to rivers draining large basins.

3.3. Application of the Model to Basins in Idaho

[29] We tested the general applicability of the broken power law using data from similar-sized basins in central Idaho. Figure 8 presents the BPL fit to the frequency distributions of daily flows from three representative gauging stations in this region. From Figure 8, it is apparent that the BPL function fits the distributions of daily flows at these sites relatively well. The χ^2_v values computed from the parameter fits of the BPL to the sites in Idaho (Table 2) are comparable to the sites in Colorado and in only a few cases is the χ^2_v much greater than 1.0, the average for Colorado.

[30] Differences in the seasonality and intensity of runoff (snowmelt versus rainfall) evidently determine how well the BPL function fits the distribution of intermediate to high flows. We investigated these hydroclimatic effects further by evaluating the annual correlation of daily flows in the Colorado and Idaho study regions. Figure 9 shows a series of annual hydrographs and corresponding 1 year autocorrelation functions for representative gauging stations in

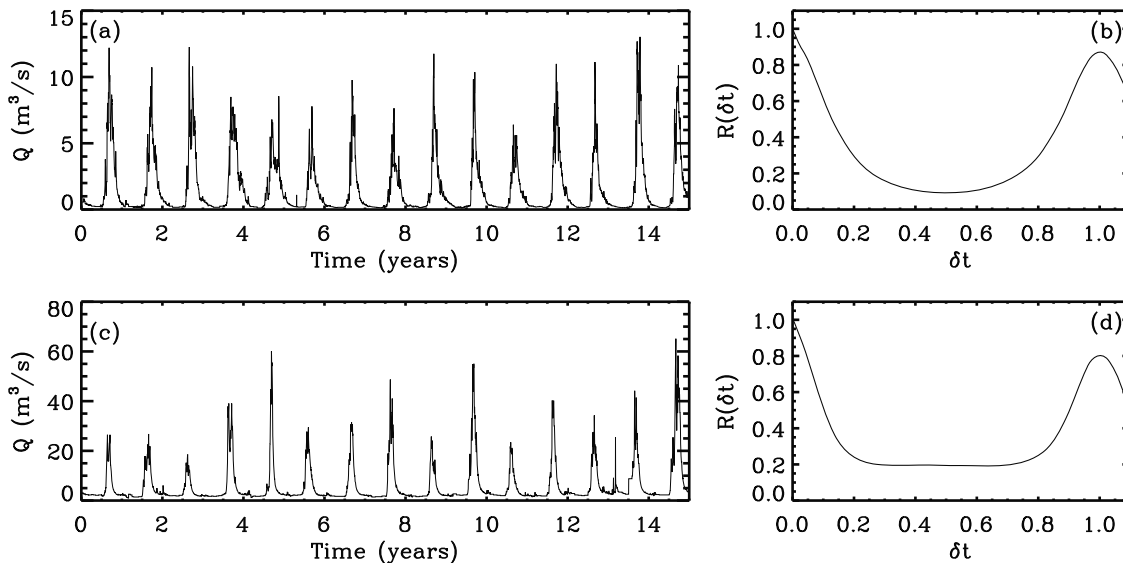


Figure 9. Fifteen year hydrograph and autocorrelation of daily flows of (a and b) North St. Vrain River, Colo, USGS station 06721500 and (c and d) MF Salmon River, Idaho, USGS station 13308500.

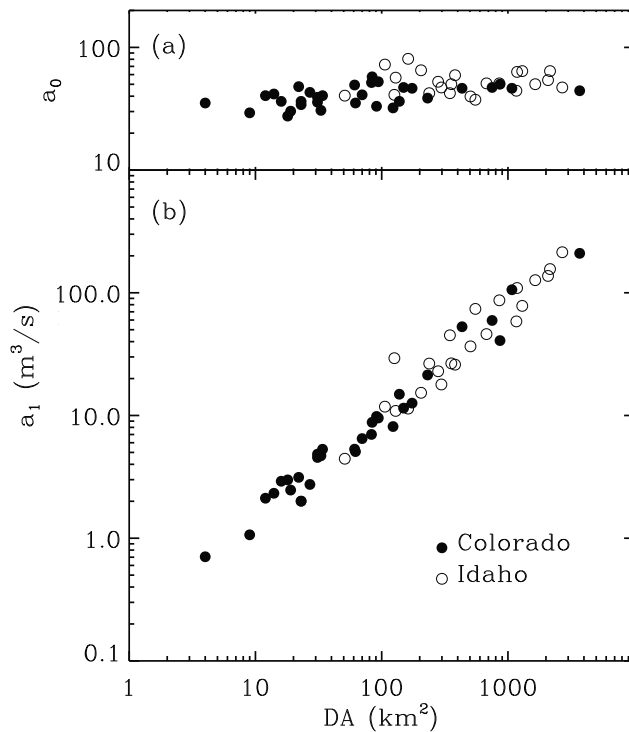


Figure 10. Parameters a_0 and a_1 of the broken power law (BPL) function versus drainage area (DA) for sites in Colorado and Idaho with 1 year autocorrelation greater than 0.70.

these two regions. The 1 year autocorrelation, $R(1y)$, is computed as

$$R(\delta t) = \frac{\int Q(t)Q(t + \delta t)dt}{\int Q^2(t)dt}, \quad (10)$$

where δt is the time shift (in this case 1 year), and $Q(t)$ is the discharge on a given day. In the examples shown in Figure 9, the autocorrelation functions exhibit a distinct peak at $\delta t = 1$. We repeated these calculations for all of the gauging stations listed in Tables 1 and 2 and found that the 1 year autocorrelation, $R(1y)$, exceeded 0.70 at all of the sites in Colorado and 72% of the sites in Idaho (Tables 1 and 2). A value of $R(1y) \sim 0.70$ might thus be used as a criterion for distinguishing between strong/weak correlations in flow and for determining in advance whether the BPL might be more applicable for fitting daily flow frequency distributions than other candidate functions.

[31] Figure 10 shows relations between the best-fit parameters of the BPL, a_0 and a_1 , and drainage area, DA, for the 55 sites in Idaho and Colorado where $R(1y) > 0.70$. The same scale is used for both a_0 and a_1 to illustrate the much greater range in a_1 compared to a_0 . The parameter values from each region broadly overlap, with the slightly higher values in Idaho reflecting larger drainage areas. We computed separate regression equations for each data set and performed an analysis of covariance (ANCOVA) to compare the slopes and intercepts of the separate relations. The ANCOVA results indicate that the slopes and intercepts of the separate relations are not significantly different ($P \gg 0.05$); hence, we combined the data to form single relations for a_0 and a_1 . Significance tests of the regression equations

(Table 4) indicate that the best-fit relations for a_0 and a_1 are statistically significant at the $P < 0.001$ level.

[32] In section 3.2 we presented a relation between a_1/DA and DA showing that the inflection point between intermediate and high flows does not scale in proportion to DA. Adding the Idaho data to that relation (Figure 11) reinforces this point and supports the conclusion made earlier that the inflection point in the frequency distribution of snowmelt flows shifts as drainage area increases. Significance tests of the regression parameters again indicate there is no significant difference in the slopes and intercepts of the separate relations ($P > 0.40$), and the relation between a_1/DA and DA is statistically significant (Table 4). Figure 11b shows that the close association between bankfull discharge, Q_{bf} , and a_1 is also maintained across the two data sets. The results in Figure 11 thus suggest that the scale-dependent changes in the frequency of intermediate to high flows, and the bankfull flow, in particular, appears to be a characteristic of these snowmelt-dominated systems.

4. Interpretation

[33] The river systems considered in this analysis represent a specific class of channels draining rugged, high-elevation areas where runoff is generated principally by snowmelt. Sediment supply to these channels is not especially high; however, the sites are all located in alluvial reaches that are relatively free of flow regulation; thus, we assume that the observed channel characteristics have evolved over time in response to the prevailing water and sediment supply. We have not assumed, however, that these processes act with the same intensity within a basin and have presented results indicating that the frequency of bankfull discharge varies with scale. In the sections that follow, we consider processes that might lead to scale-dependent changes in the frequency of bankfull flow.

4.1. Attenuation and Storage

[34] Peak discharges generally attenuate as they move through the channel network, and it has been suggested in several studies that storage within the channel and adjacent floodplain can alter the magnitude and frequency distribution of peak flows downstream [Wolff and Burges, 1994; Woltemade and Potter, 1994; Gupta and Dawdy, 1995]. In a more recent study, Dodov and Fofoula-Georgiou [2005] found that, in contrast to our results, the frequency of

Table 4. Summary and Statistics for Regression Between the Parameters, a_0 and a_1 , of the Broken Power Law and Drainage Area for Basins in Colorado and Idaho^a

| Y | No. | a | b | r ² | P |
|------------|-----|---------------|----------------|----------------|----------|
| a_0 | 64 | 30.2 (2.7) | 0.078 (0.017) | 0.261 | <0.0001 |
| a_1 | 64 | 0.164 (0.029) | 0.871 (0.033) | 0.917 | <<0.0001 |
| a_0^b | 55 | 30.6 (2.6) | 0.075 (0.016) | 0.289 | <<0.0001 |
| a_1^b | 55 | 0.197 (0.023) | 0.858 (0.022) | 0.966 | <<0.0001 |
| a_1/DA | 64 | 0.164 (0.029) | -0.129 (0.033) | 0.196 | 0.0002 |
| a_1/DA^b | 55 | 0.197 (0.023) | -0.142 (0.022) | 0.437 | <<0.0001 |

^a(Figures 10 and 11): No. is the number of basins considered, and a and b are parameters of the power law relation, $Y = aX^b$; values shown in parenthesis are standard errors of the regression coefficients, SE_a and SE_b ; r^2 is the coefficient of determination; and P is the significance probability.

^bRegression based on the data of Colorado and Idaho combined excluding sites with 1 year autocorrelation less than 70%.

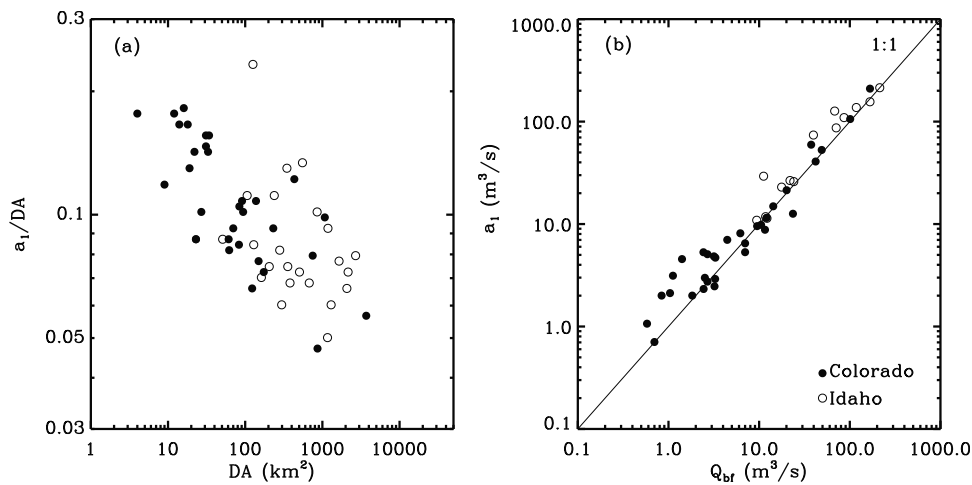


Figure 11. (a) Relation between the parameter a_1 of the broken power law function, normalized by drainage area (DA), and DA; (b) relationship between a_1 and Q_{br} across basins in Colorado and Idaho excluding sites with 1 year autocorrelation less than 0.70.

flows exceeding Q_{br} increased downstream. These authors suggested that as slope decreases and channels become more sinuous downstream, the added resistance to flow over point bars and vegetated banks decreases the celerity of flood waves, thus, generating higher stages for longer periods of time.

[35] We evaluated the potential influence of attenuation and storage on flow frequency by comparing continuous records of discharge at two gauges in the Williams Fork basin in Colorado. The two main tributaries of the Williams Fork are gauged just above the point where they join, and the main stem is gauged another 5 km downstream where the river meanders through a valley with a wide floodplain. We hypothesized that if changes in slope and sinuosity affect the translation of peak flows, we should see clear differences in the hydrographs at upstream and downstream gauges. We selected 1995, a year of high runoff, and compared simultaneous 15 min records of discharge at the upstream and downstream gauges, normalizing each value of discharge by drainage area to account for differences in scale. Figure 12 shows continuous records for 1995, in which there were two peaks, and the bankfull discharge was exceeded frequently at both gauges. It appears that in the period leading up to the first peak (days 30–40, Figure 12b) unit discharges at the two gauges tracked each other closely. If there was much attenuation in the reach between the gauges, it was probably offset by inputs from tributaries. During the peak (days 45–55, Figure 12b), unit discharges at the downstream gauge were generally lower than the upstream gauge, suggesting there was some storage associated with the transition to overbank flow. The hydrograph for the second peak (Figure 12c) exhibits a different pattern, with little difference in discharge during the peak, but higher discharges at the downstream gauge on the falling limb. The crossover in upstream-downstream discharge relations is consistent with the hypothesis that the floodplain will act first as a sink, then as a source for downstream reaches, however, the differences here are probably too small to conclude that floodplain storage has much influence on the frequency of bankfull flow.

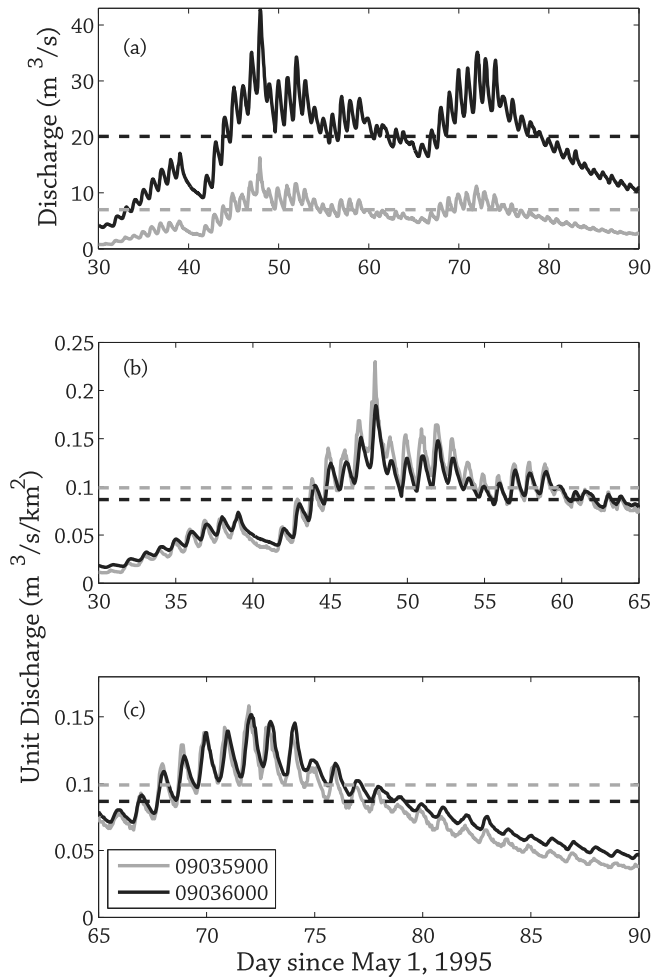


Figure 12. Records of instantaneous discharge, May–July 1995, for two gauges in the Williams Fork watershed. Gauge 09035900 drains 70.3 km² and is located on a tributary with limited floodplain area. Gauge 09036000 drains 231 km² and is located in a wide valley with an extensive floodplain area. Dashed lines indicate bankfull discharge.

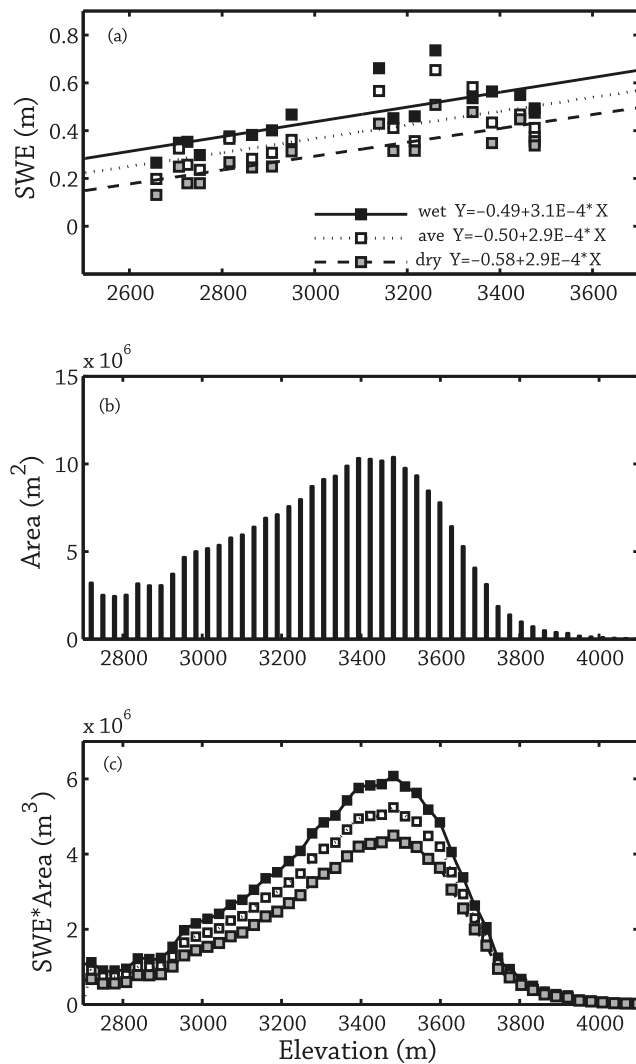


Figure 13. (a) Relationship between snow water equivalent (SWE) and elevation for 19 stations in the upper Colorado River region during wet, average, and dry conditions; (b) frequency distribution of elevation in the Williams Fork basin (drainage area = 231 km²) derived from a 10 m digital elevation model; and (c) volumetric water content for wet, average, and dry years.

4.2. Spatial Variability in Runoff

[36] We explored another possibility that the differences in bankfull flow frequency are related to spatial variations in the volume of runoff produced by snowmelt. Data from snow telemetry (SNOTEL) stations in the upper Colorado River basin indicate that the April 1 snow water equivalent (SWE) increases linearly with elevation (Figure 13a). However, the terrain over which snow is distributed is very irregular; thus, the spatially averaged water content becomes a more complex function of basin hypsometry (area above a given elevation). Figure 13b shows the relation between area and elevation in the drainage basin of the Williams Fork, derived from a digital elevation model. This distribution exhibits a pattern typical of high-relief terrain where the relation between area and elevation increases more or less continuously to a point (~3500 m) then drops off

sharply. If we combine the relations for SWE in Figure 13a with the distribution of area in Figure 13b, we get relations for volumetric water content in wet, average, and dry years, as shown in Figure 13c. These results suggest that in alpine-subalpine basins in Colorado, the majority of runoff is derived from areas that lie between about 3100 and 3700 m in elevation. At elevations higher and lower than this, the volume of water held in the snowpack decreases, and differences between wet and dry years diminishes. These results suggest that in small headwater basins the volume of runoff produced in any given year is never much higher or lower than the average. Consequently, the geomorphic processes which determine the bankfull hydraulic geometry act with more or less the same intensity each year, leading to a higher frequency of bankfull flows. As drainage area increases, the amount of runoff generated per unit area decreases, and the differences between wet and dry years become more pronounced. In addition, at elevations between about 3000 and 2500 m, streams pass beyond the glacial limit and into more sparsely vegetated areas where the sediment supply increases. Results from our previous studies of these river systems [Mueller and Pitlick, 2005; Pitlick et al., 2008; Segura, 2008] indicate that, as drainage area and sediment supply increase, channel and bed material properties evolve downstream to produce a bed that is less armored and thus able to sustain low to moderate levels of bed load transport over a wider range of discharges. We suggest, therefore, that the upstream-downstream transitions in flow frequency in these river systems reflect not only the spatial variations in runoff but also the way in which the channel geometry and bed material evolve downstream to convey higher sediment loads with proportionally less water.

5. Conclusions

[37] The goal of this paper was to develop a regional flow frequency model that could then be used to examine the scaling behavior of channel-forming discharges. The results indicate that the frequency distribution of daily flows in snowmelt-dominated river systems can be reproduced relatively well with a two-parameter broken power law distribution (BPL). The BPL model appears to be most applicable to stream systems with predictable patterns of runoff, i.e., daily flows with 1 year autocorrelation above 0.7. The two parameters of the BPL are both strongly correlated with drainage area, and based on these correlations, a regional model capable of predicting the frequency distribution of intermediate to high flows was formulated. According to the derived model, the parameter which defines the inflection point in the BPL distribution, a_1 , increases with drainage area, DA; however, we find that the relation between a_1 and DA is nonlinear. The shift in the inflection point apparently reflects systemwide changes in hydrology which lead to a downstream decrease in the frequency of high flows (and an increase in the frequency of intermediate flows). The relation between a_1 and DA thus suggests that daily flows in snowmelt-dominated stream systems exhibit multiscaling behavior. We also find that a_1 is strongly and positively correlated to the bankfull discharge, Q_{bf} . When coupled with the relation between a_1 and DA, this last result indicates that there is a scale-dependent change in the frequency of channel-forming flows.

[38] Our analysis of the timing and translation of diurnal variations in flow in the Williams Fork basin ($DA = 231 \text{ km}^2$) suggests that the volumes of water that go into floodplain storage during the height of snowmelt runoff are relatively small in comparison to discharges conveyed within the channel. It appears that, in these types of river systems, there is an upper limit to the intensity of runoff produced by snowmelt; hence, attenuation due to within-channel or overbank storage does not strongly influence the distribution of daily flows. These effects would likely be more pronounced in hydroclimatic regions where floods are produced by rainfall (or rain on snow) and where floodplains are inundated to greater depths. We suggest that a better case can be made for the influence of snow cover and topography, which combine to generate a peak in snowmelt runoff at intermediate drainage areas. Our hypothesis warrants further testing, but we infer from the analysis of snow cover and topography, plus results from previous work on these same river systems, that channels evolve downstream to produce widths, depths, and slopes that can sustain low to moderate levels of bed load transport over a wider range of discharges.

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